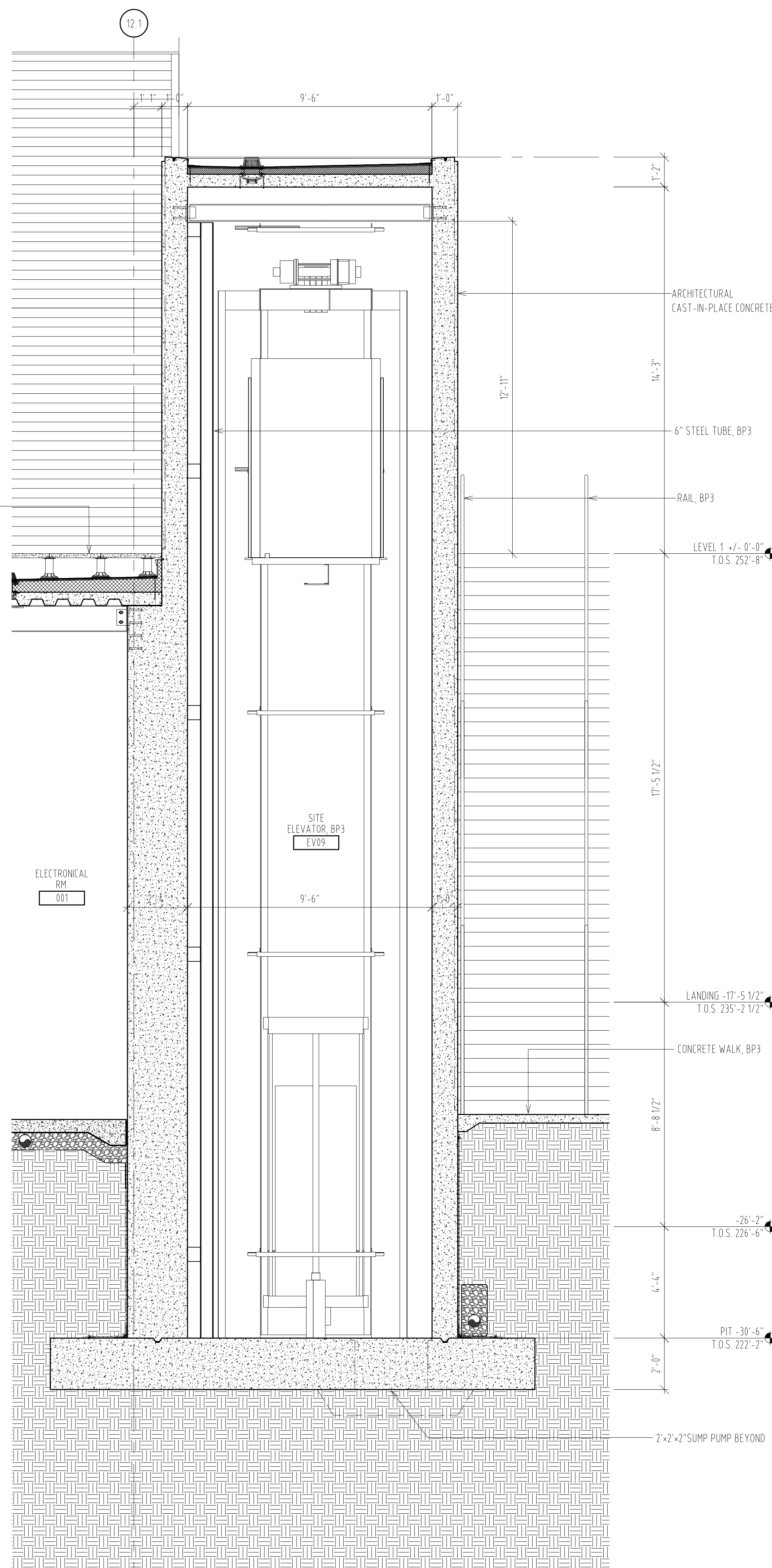
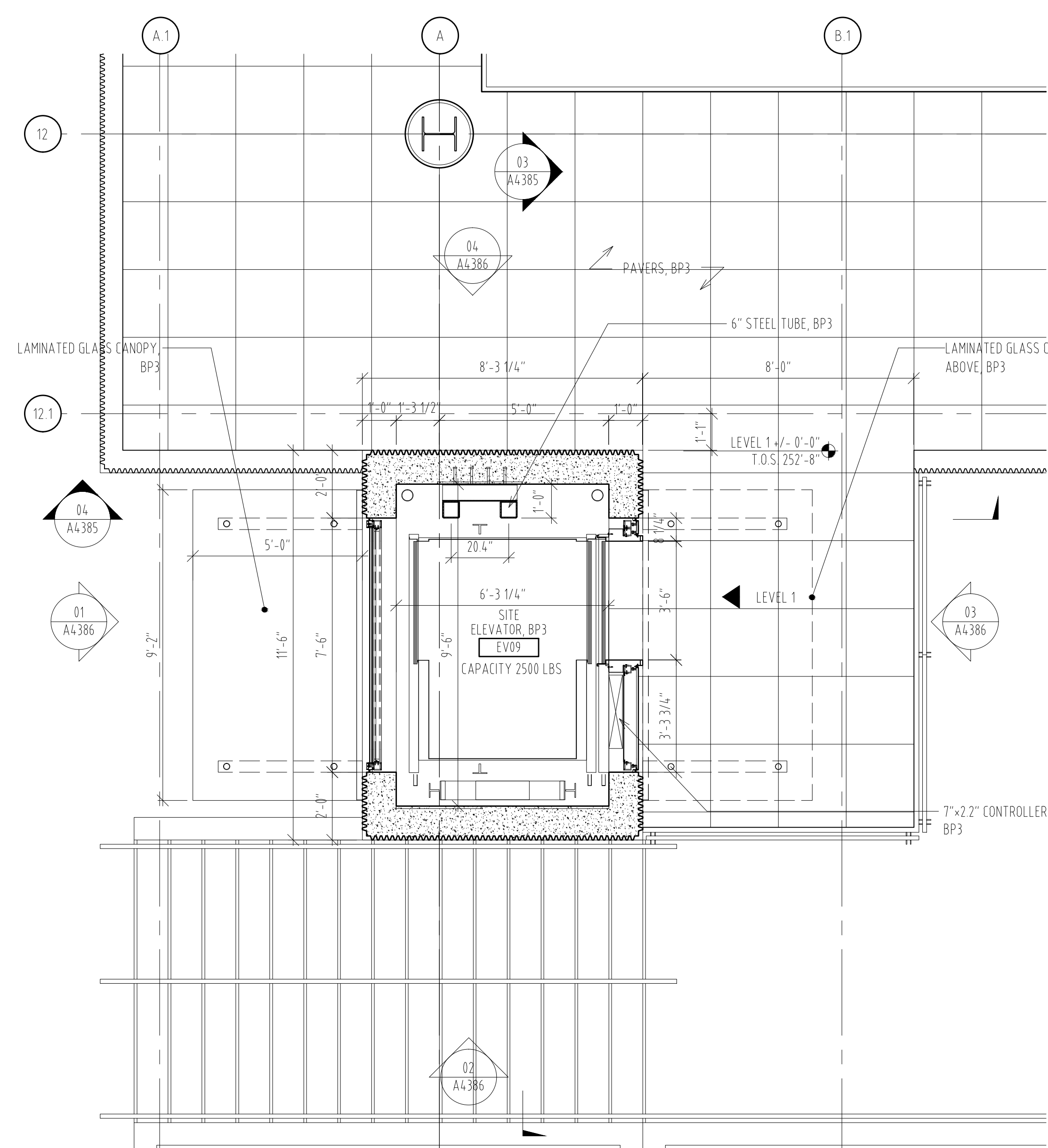


6

1) FOR STRUCTURAL INFORMATION, REFER TO STRUCTURE DWG

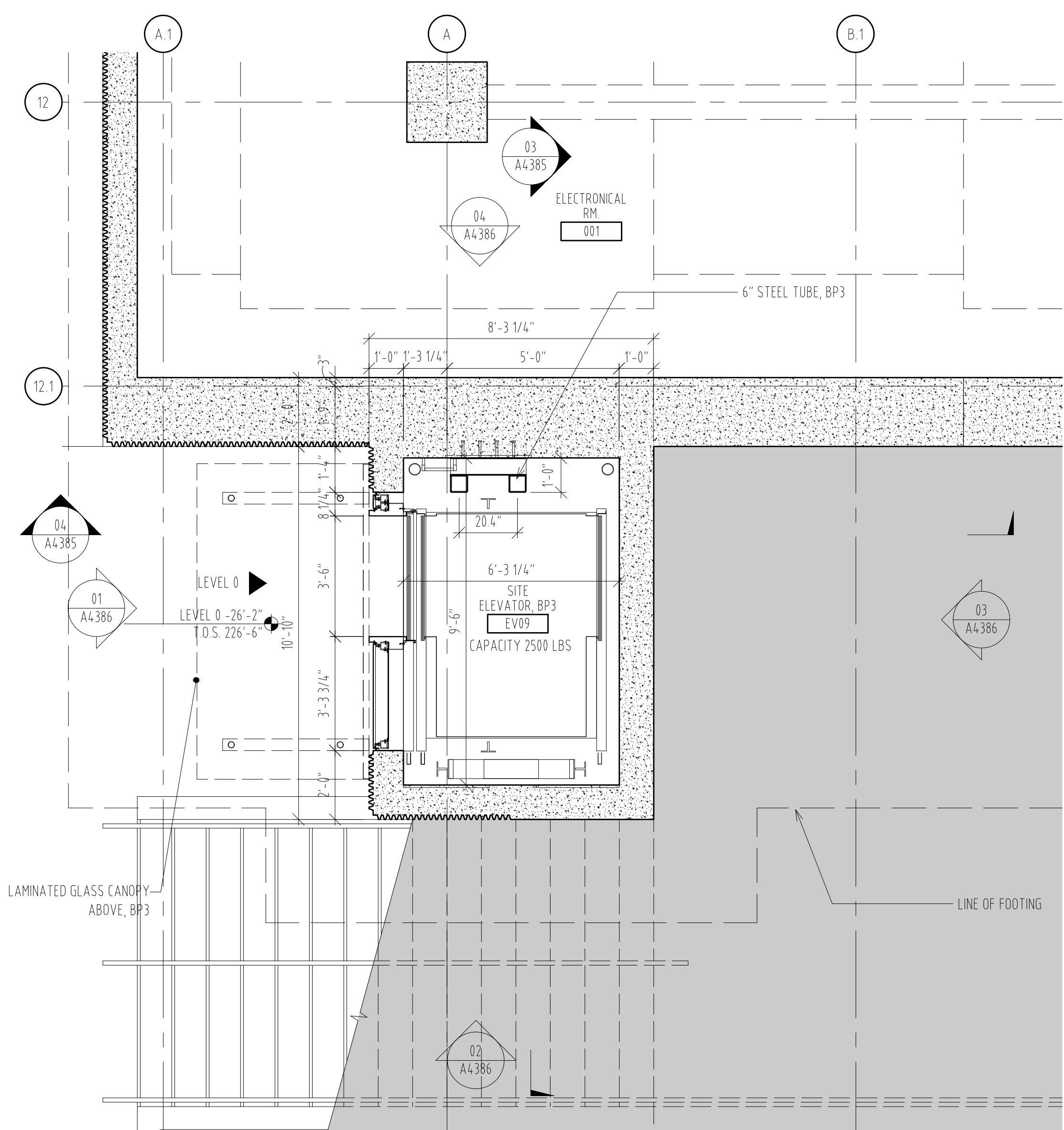


SITE ELEVATOR- SECTION	03
SCALE: 3/8" = 1'-0"	



SITE ELEVATOR- PLAN- LEVEL 1
SCALE: 3/8" = 1'-0"

-GENERAL NOTE
1) FOR STRUCTURAL INFORMATION REFER TO STRUCTURE DWG

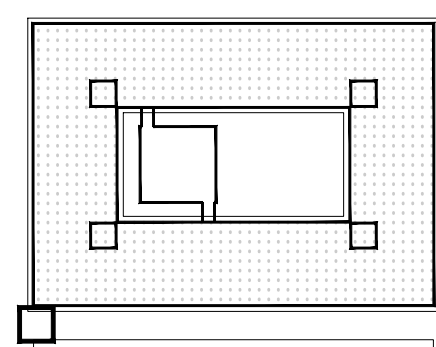


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SCALE: 3/8" = 1'-0"

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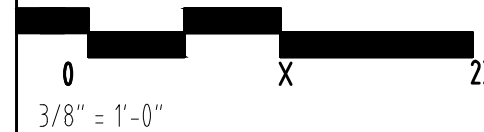
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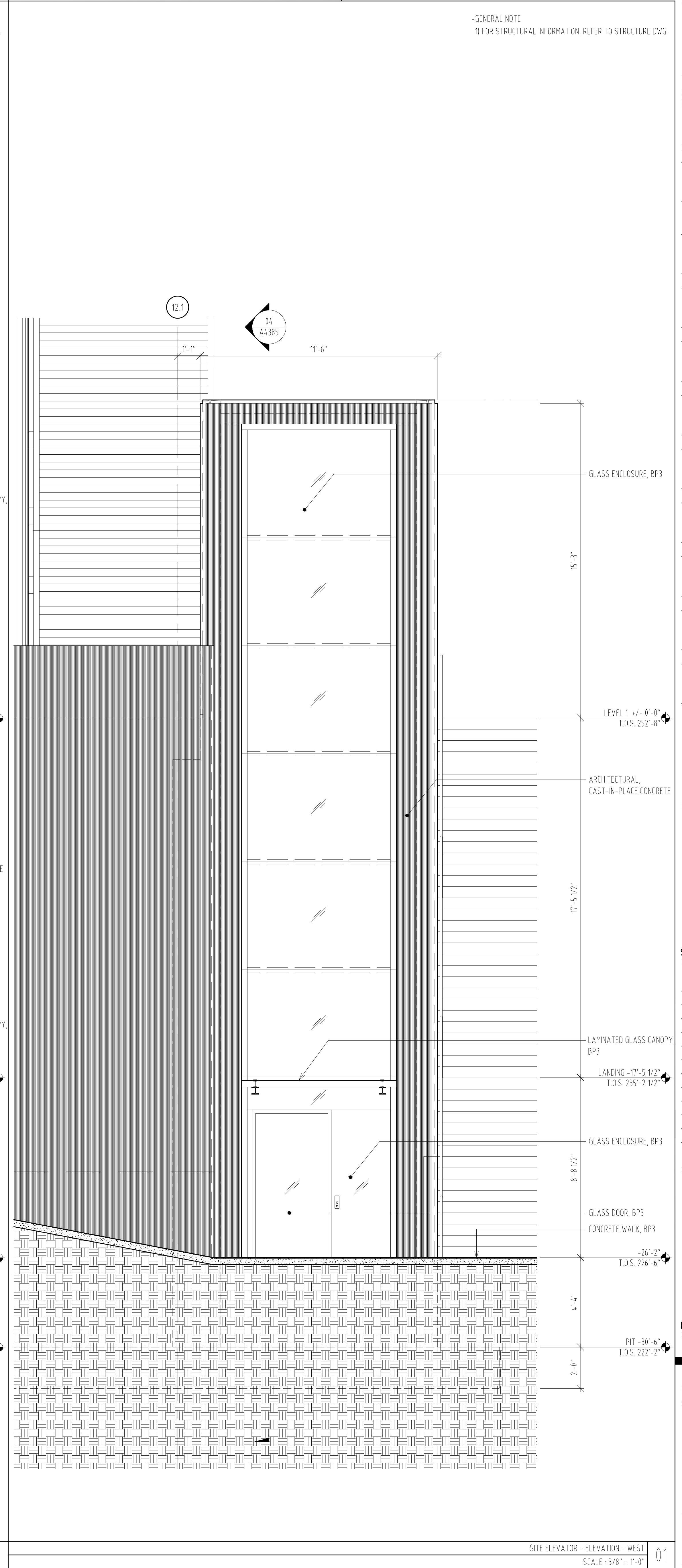
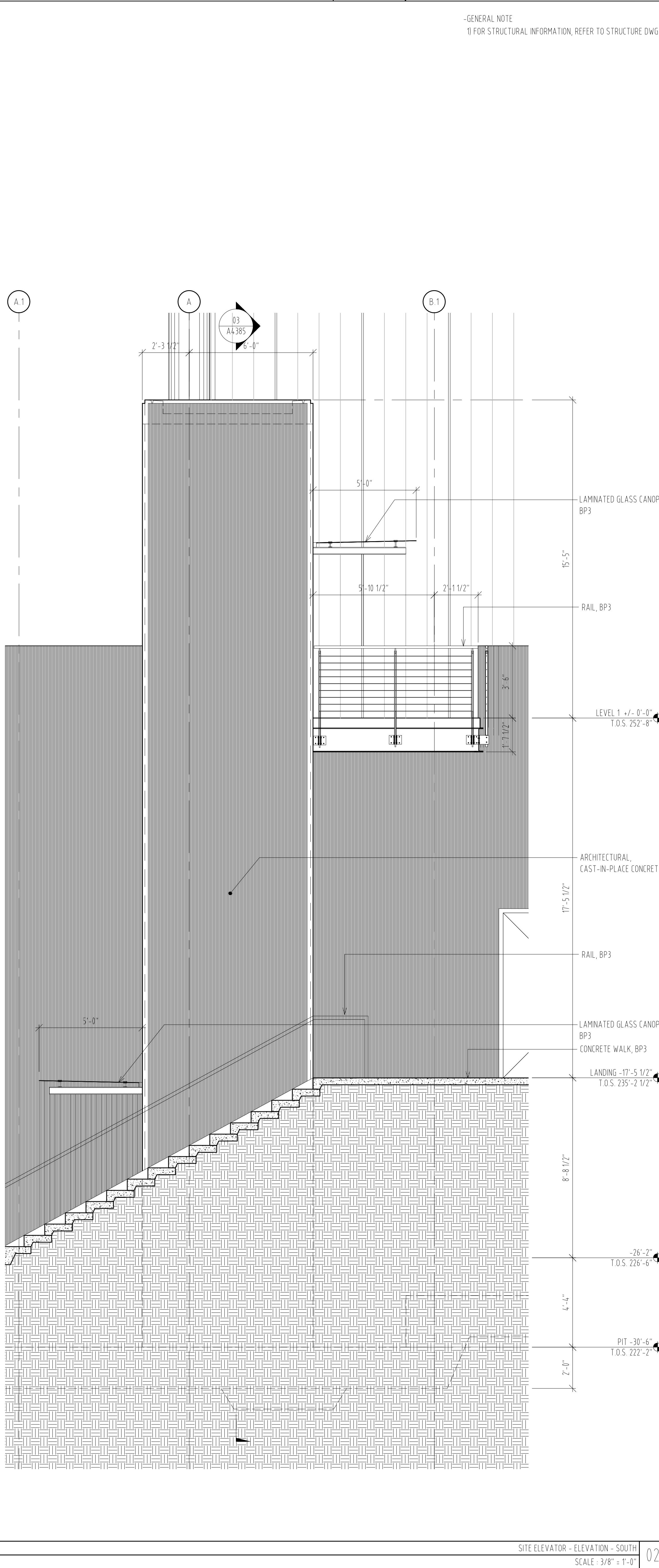
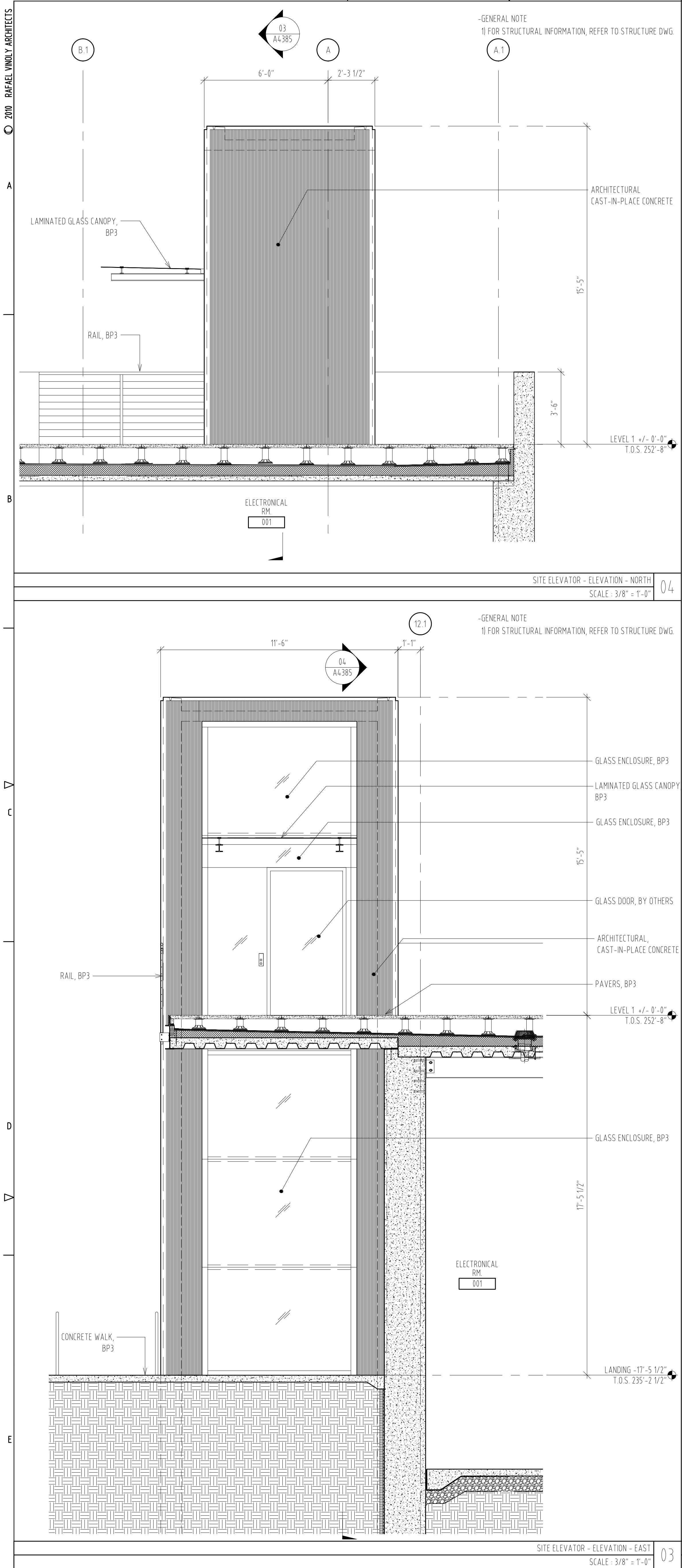
KEY PLAN & NORTH SIGN

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ENLARGED PLANS, SECTIONS
SITE ELEVATOR

SHEET TITLE :

A4385
SHEET NUMBER :



USC BUSINESS PARTNERSHIP FOUNDATION
ON BEHALF OF:
UNIVERSITY OF SOUTH CAROLINA
DARLA MOORE SCHOOL OF BUSINESS CONSTRUCTION
STRUCTURE

PROJECT NAME:
DARLA MOORE SCHOOL OF BUSINESS CONSTRUCTION
STRUCTURE

OSE PROJECT NUMBER:
H27-6069-AC-2

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1	2011.12.09	2	2011.11.30	3	2011.11.16
4	2011.10.25	5	2011.09.21	6	2011.08.09

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REFER TO GRAPHIC SCALE

0 1 2X
3/8" = 1'-0"

ENLARGED ELEVATIONS
SITE ELEVATOR

SHEET TITLE:
A4386
SHEET NUMBER:



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ON BEHALF OF
USC CAMPUS PLANNING & CONSTRUCTION

PROJECT NAME:
UNIVERSITY OF SOUTH CAROLINA
DARLA MOORE SCHOOL OF BUSINESS CONSTRUCTION
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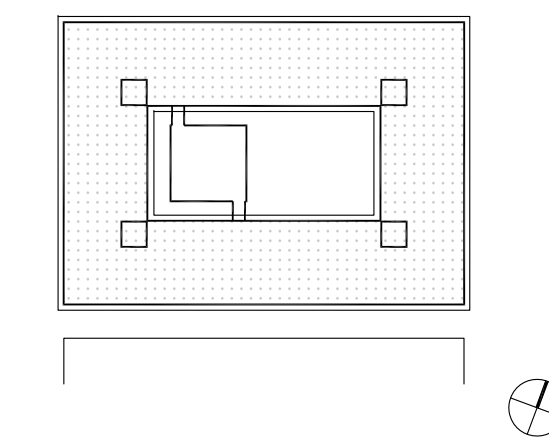
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STRUCTURE
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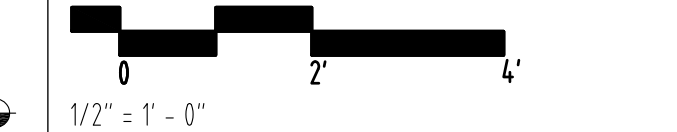
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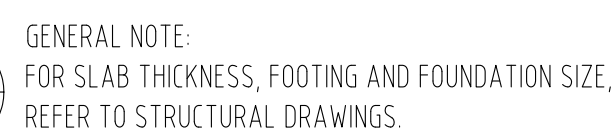
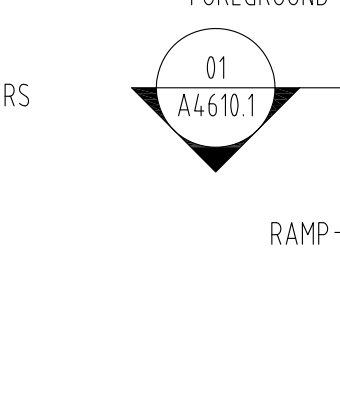
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EWS-0.1 - ARCHITECTURAL
CONCRETE WALL
WEST FACADE

SHEET TITLE :

1 A4610.0
SHEET NUMBER :






PHASE :

KEY PLAN & NORTH SIGN

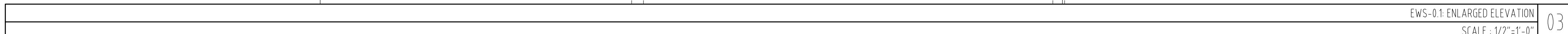
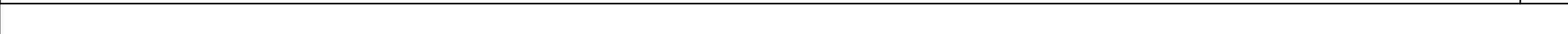
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0 2' 4'

$1/2" = 1' - 0"$

SHEET TITLE :
A4610.1
SHEET NUMBER :



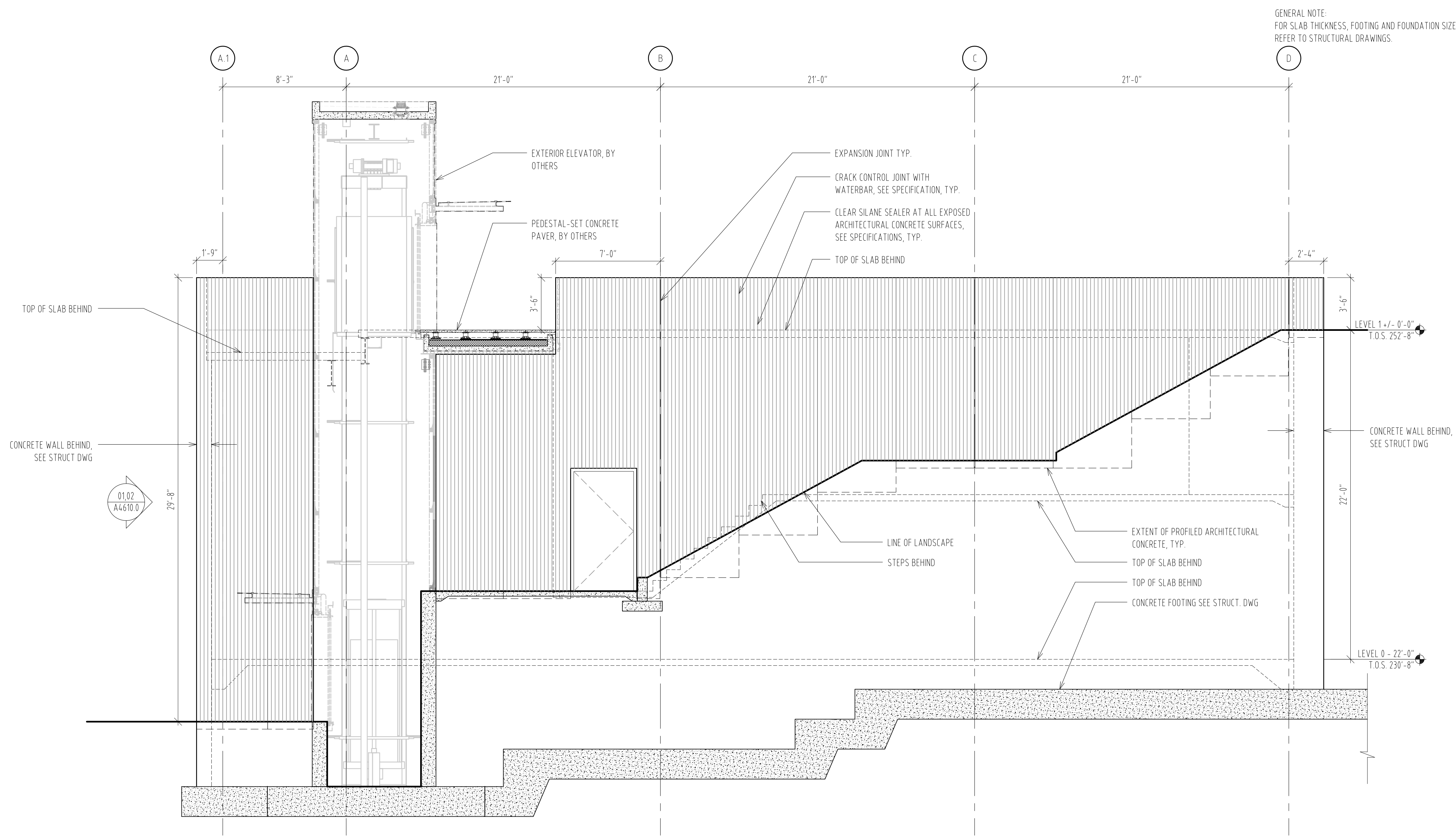
PHASE :

0 2' 4'

$1/2'' = 1' - 0''$

• **Prevalence** is the proportion of the population with a disease at a particular point in time.

SHEET TITLE :
A4610.2
SHEET NUMBER :



GENERAL NOTE:
FOR SLAB THICKNESS, FOOTING AND FOUNDATION SIZE,
REFER TO STRUCTURAL DRAWINGS.

EWS-0.1 ENLARGED ELEVATION - SOUTH WALL
SCALE: 1/4"=1'-0"

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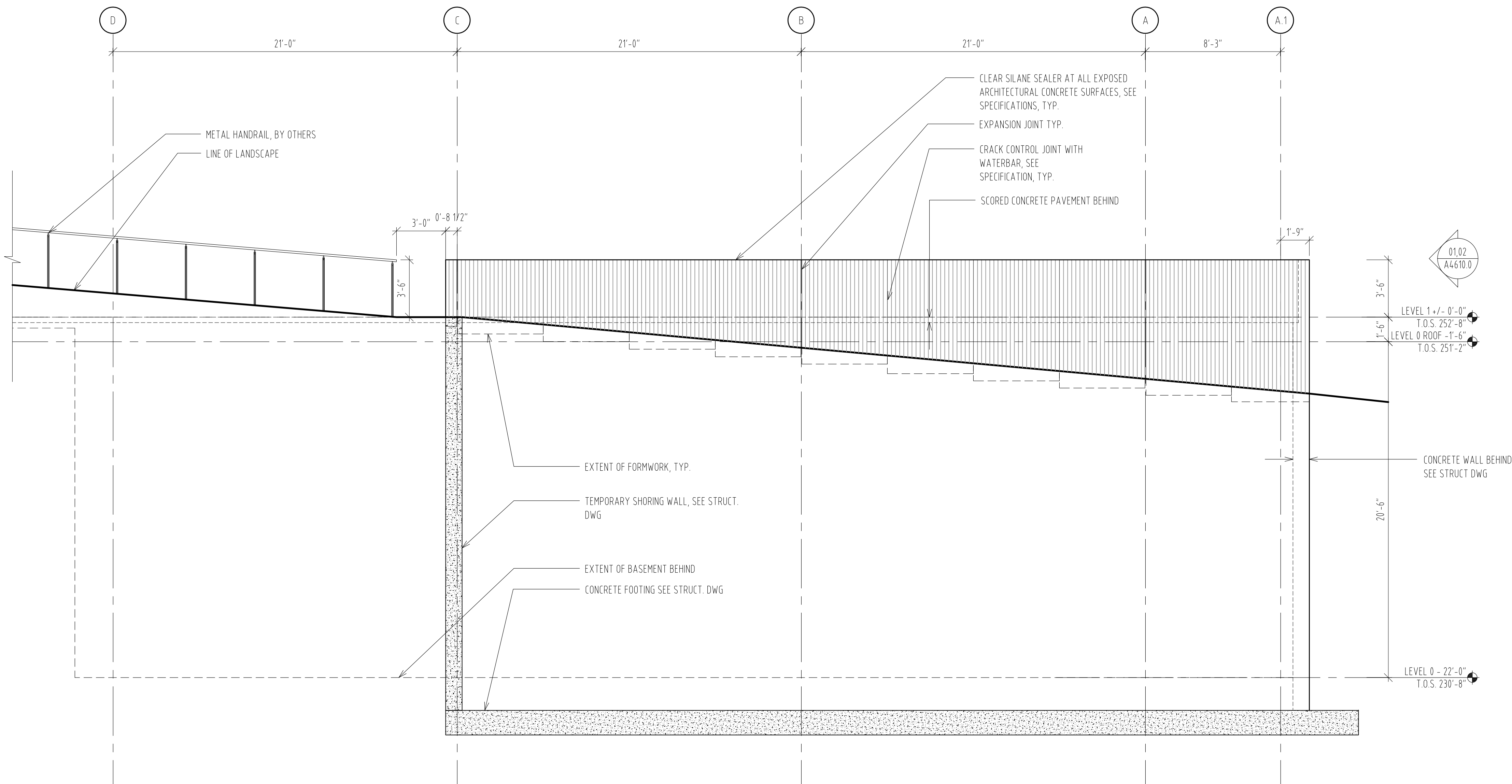
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DECEMBER 09, 2011



EWS-0.1 ENLARGED ELEVATION - NORTH WALL
SCALE: 1/4"=1'-0"

PHASE:

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ISSUE NO.	ISSUE DATE	ISSUE NO.	ISSUE DATE	ISSUE NO.	ISSUE DATE
01	2011.12.09	02	2011.11.30		

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EWS-0.1 - ARCHITECTURAL
CONCRETE WALL
NORTH & SOUTH FACADE

SHEET TITLE:

A4610.4

SHEET NUMBER:

GENERAL	CODE/DESIGN CRITERIA (CONTINUED)	CAST-IN PLACE CONCRETE	STRUCTURAL STEEL
1. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS PRIOR TO STARTING CONSTRUCTION AND THE OWNER'S REPRESENTATIVE SHALL BE NOTIFIED OF ANY DISCREPANCIES OR INCONSISTENCIES.	5. MAXIMUM ESTIMATED DEFLECTIONS (IN INCHES) ARE AS FOLLOWS: LIVE LOAD DEAD + LIVE LOAD ROOF MEMBERS: L/360 L/240 FLOOR MEMBERS: L/360 L/240 WHERE: L = SPAN LENGTH (IN INCHES) BETWEEN CENTERLINES OF SUPPORTS, (FOR CANTILEVERS, L IS TWICE THE LENGTH OF THE CANTILEVER).	1. CONCRETE SHALL HAVE THE FOLLOWING MINIMUM SPECIFIED 28-DAY COMPRESSIVE STRENGTH: 1.1 NORMAL WEIGHT STRUCTURAL CONCRETE: FOOTINGS, PEDESTALS 4,000 PSI FOUNDATION BASEMENT, RETAINING WALLS 4,000 PSI SLABS-ON-GRADE 4,000 PSI SHEAR WALLS SEE SHEAR WALL ELEVATIONS 1.2 LIGHTWEIGHT STRUCTURAL CONCRETE: (115-118 PCF UNIT WEIGHT) SLABS ON COMPOSITE STEEL DECK TOPPING SLABS 3,500 PSI 4,000 PSI 2. BASE GROUT SHALL BE NON-SHRINK GROUT CONFORMING TO ASTM C1107, USE NON-METALLIC GROUT WITH MINIMUM STRENGTH EQUAL TO 2 TIMES THE SPECIFIED 28-DAY COMPRESSIVE STRENGTH OF THE ASSOCIATED FOUNDATION. 3. NO CALCIUM CHLORIDE SHALL BE USED IN ANY CONCRETE. 4. THE CONTRACTOR SHALL SUBMIT FOR APPROVAL DETAILED DRAWINGS SHOWING THE LOCATION OF ALL CONSTRUCTION JOINTS, CURBS, SLAB DEPRESSIONS, SLEEVES, OPENINGS, ETC. IN ALL CONCRETE WORK. STOP IN WORK SHALL BE MADE AT THE THIRD POINT OF SPAN WITH VERTICAL BULKHEADS. THERE SHALL BE NO HORIZONTAL CONSTRUCTION JOINTS IN SLABS AND BEAMS. PROVIDE KEYS IN SLABS OR BEAMS AT CONSTRUCTION JOINTS. ALL REINFORCING SHOWN ON THE DRAWINGS SHALL BE CONTINUOUS THROUGH CONSTRUCTION JOINTS. 5. ALL CONSTRUCTION JOINTS SHALL BE WIRE BRUSHED, CLEANED, AND COATED WITH CONCRETE BONDING AGENT IMMEDIATELY PRIOR TO PLACING NEW CONCRETE. 6. FORMS SHALL BE CAMBERED AS INDICATED. CAMBER SHALL NOT BE ACHIEVED BY ADDING THICKNESS TO SLAB OR BEAMS. 7. REFER TO ARCHITECTURAL DRAWINGS FOR CONCRETE FINISHES, REVEALS, AND/OR PATTERNS. 8. HANGER INSERTS IN CONCRETE SLAB SHALL BE PLACED SO THAT 1" CONCRETE COVER OCCURS BETWEEN INSERT AND TOP OF SLAB. 9. CONDUIT SHALL BE PLACED UNDER THE SLAB. NO CONDUITS SHALL BE PERMITTED TO RUN HORIZONTALLY IN COMPOSITE OR FORMED SLABS OR SLABS ON GRADE. 10. SLEEVE PLUMBING OPENINGS IN CONCRETE WALLS AND SLABS BEFORE PLACING CONCRETE. ADJUST REINFORCING AT SLEEVES TO PROVIDE REQUIRED COVER TO REINFORCING. CORING IS NOT PERMITTED IN FLOOR SLABS, ROOF SLABS, COLUMNS, AND WALLS UNLESS PERMITTED BY OWNER'S REPRESENTATIVE. 11. CONTRACTOR SHALL PROVIDE PROPER STORAGE FACILITIES FOR CONCRETE TEST CYLINDERS TO MAINTAIN CYLINDERS BETWEEN 60 DEGREES AND 85 DEGREES F AND IN A MOIST CONDITION.	1. ALL DETAILING, FABRICATION, AND ERECTION SHALL CONFORM TO THE "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND THE ALSO "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES", CURRENT EDITION, STRUCTURE 2. ALL STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING (UNLESS NOTED OTHERWISE): 2.1 ASTM A36 FOR ANGLES, PLATES, & WHERE NOTED A36, UNO. 2.2 ASTM A992 OR A572, GRADE 50 (F = 50 KSI) FOR ALL WIDE FLANGE SECTIONS AND CHANNELS. 2.3 ASTM A500, GRADE B FOR STRUCTURAL STEEL TUBING (NOTED HSS...). 2.4 ASTM A53, TYPE S, GRADE B FOR STRUCTURAL STEEL PIPE (NOTED PIPE) 2.5 WHERE STEEL MATERIAL IS NOT INDICATED, OR NOTED ABOVE STEEL SHALL BE, HEREIN OR ON DRAWING. 3. BOLTS, ANCHOR RODS, AND HEADED STUDS: 3.1 ALL BOLTS, NUTS, AND WASHERS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A305 OR ASTM A590. ALL BOLTS SHALL BE A MINIMUM 3/4" DIAMETER UNLESS NOTED OTHERWISE. 3.2 ANCHOR RODS SHALL CONFORM TO ASTM F1554, GRADE 36, UNLESS NOTED OTHERWISE. 3.3 HEADED STUDS SHALL BE 3/4" DIAMETER, UNLESS NOTED OTHERWISE, AND SHALL CONFORM TO AISC D1.1. LENGTH OF STUD SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE ON THE DRAWING. COMPOSITE BEAMS: 4-7/8" LONG HEADED STUDS (FOR 6-1/4" SLABS) MISCELLANEOUS EMBEDS: SEE CONTRACT DRAWINGS 4. ALL WELDING ELECTRODES SHALL BE E70XX EXCEPT ELECTRODES FOR WELDING METAL DECK SHALL BE E60XX. 5. CONNECTIONS SHALL BE DETAILED BASED ON THE DESIGN INFORMATION PROVIDED IN THE CONTRACT DOCUMENTS. DEVIATION FROM THE CONNECTION DETAILS DEPICTED IN THE CONTRACT DOCUMENTS SHALL NOT BE PERMITTED WITHOUT WRITTEN PERMISSION FROM THE STRUCTURAL ENGINEER. 5.1 CONNECTION FORCES PROVIDED ARE LRFD FORCES. CONNECTIONS SHALL BE DESIGNED WITH THE LRFD PROVISIONS OF THE MANUAL. 5.2 STANDARD SHEAR CONNECTIONS SHALL UTILIZE HIGH-STRENGTH BOLTS IN BEARING-TYPE CONNECTIONS WITH THEREAS INCLUDES IN THE SHEAR PLANE(S). STANDARD SHEAR CONNECTIONS SHALL BE DETAILED AS DOUBLE-ANGLE, SINGLE-PLATE, SINGLE-ANGLE, OR TEE CONNECTIONS IN ACCORDANCE WITH CONNECTION TABLES IN THE "STEEL CONSTRUCTION MANUAL", 13TH EDITION, PART 10. 5.3 FOR WELDED CONNECTIONS, USE PREQUALIFIED WELDED JOINTS IN ACCORDANCE WITH AISC AND THE STRUCTURAL WELDING CODE OF THE AMERICAN WELDING SOCIETY. NON-PREQUALIFIED JOINTS SHALL BE QUALIFIED PRIOR TO FABRICATION. 5.4 STRENGTH (LRFD) DESIGN REACTIONS SHALL BE AS SHOWN ON THE STRUCTURAL DRAWINGS. IF REACTIONS ARE NOT SHOWN: FOR BEAMS W10 AND SMALLER, PROVIDE CONNECTION WITH A MINIMUM 1/4" RIP SHEAR CAPACITY; FOR BEAMS W12 AND LARGER, THE FACTORED DESIGN REACTION SHALL BE HALF OF THE MAJOR MOMENT ABOUT THE WEAK AXIS AS LABULATED IN THE "STEEL CONSTRUCTION MANUAL", 13TH EDITION, PART 3. 5.5 STEEL CONNECTIONS NOT SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED AND DETAILED BY THE FABRICATOR'S ENGINEER. DETAILING SHALL BE PERFORMED USING RATIONAL ENGINEERING DESIGN AND STANDARD PRACTICE IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. THE GENERAL DETAILS SHOWN ON THE DRAWINGS ARE CONCEPTUAL ONLY AND DO NOT INDICATE THE REQUIRED NUMBER OF BOLTS OR WELD SIZES, UNLESS NOTED OTHERWISE. 5.6 THE MINIMUM NUMBER OF BOLTS PER CONNECTION SHALL BE TWO (2). 5.7 MINIMUM FILLET WELDS SHALL COMPLY WITH AISC BUT SHALL NOT BE LESS THAN 3/16", UNLESS NOTED OTHERWISE.
2. ALL DIMENSIONS TO TAKE PRECEDENCE OVER SCALE SHOWN ON PLANS, SECTIONS AND DETAILS.	6. NO PROVISIONS HAVE BEEN MADE FOR FUTURE HORIZONTAL EXPANSION. FUTURE VERTICAL EXPANSION IS LIMITED TO LEVEL 5 PAVILION/PERGOLAS IN SPECIFIC AREAS NOTED ON PLAN.	CONCRETE MASONRY 1. CONCRETE MASONRY WORK SHALL CONFORM TO ACI 530, BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES AND ACI 530.1, SPECIFICATION FOR MASONRY STRUCTURES. 2. MINIMUM COMPRESSIVE STRENGTH OF CONCRETE MASONRY SHALL BE FM = 1,500 PSI. 3. CONCRETE MASONRY UNITS SHALL CONFORM TO THE FOLLOWING: 3.1 LOAD BEARING AND NON-LOAD BEARING MASONRY WALLS: ASTM C90 HOLLOW UNITS 3.2 CONCRETE BUILDING BRICK: ASTM C57 4. MORTAR SHALL CONFORM TO ASTM C270: 4.1 TYPE "S" MORTAR, UNO. 5. GROUT FOR ALL MASONRY SHALL CONFORM TO ASTM C476. SUBMIT GROUT MIX DESIGNS INCLUDING MANUFACTURER'S CERTIFICATION FOR MATERIALS USED PRIOR TO THE START OF ANY MASONRY WORK. 5.1 PROVIDE FINE GROUT IN GROUT SPACES LESS THAN 2" IN ANY HORIZONTAL DIMENSION OR WHERE CLEARANCE BETWEEN REINFORCING AND MASONRY IS LESS THAN 3/4". 5.2 PROVIDE COURSE GROUT IN SPACES 2" OR GREATER IN ALL HORIZONTAL DIMENSIONS PROVIDED THE CLEARANCE BETWEEN REINFORCING AND MASONRY IS GREATER THAN 3/4". 5.3 THE MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS FOR ALL GROUT SHALL BE 3,000 PSI, UNLESS NOTED OTHERWISE. 6. ALL TESTING AND INSPECTION OF MASONRY WORK SHALL BE AS SPECIFIED IN ACI 530.1. 6.1 WALLS: INSPECT EACH SECTION OF WALL AND VERIFY REINFORCEMENT PLACEMENT PRIOR TO GROUTING OPERATIONS. VERIFY THAT VERTICAL CELLS AND BOND BEAMS TO RECEIVE GROUT ARE CLEANED OUT TO RECEIVE GROUT. 6.2 GROUT: OBSERVE GROUT OPERATIONS TO INSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS. 7. FOR BRICK EXPANSION AND CONTROL JOINTS SEE ARCHITECTURAL DRAWINGS. 8. CMU CONTROL JOINTS SHALL BE LOCATED USING THE FOLLOWING GUIDELINES: 8.1 UNLESS SPECIFICALLY NOTED, CONTROL JOINT SPACING IN STRAIGHT WALLS SHALL BE THE LESSER OF ONE-AND-ONE-HALF TIMES THE WALL HEIGHT (1-1/2 x H) AND 25' AT CORNERS AND INTERSECTIONS. LOCATE CONTROL JOINT WITHIN ONE-HALF (1/2) STRAIGHT WALL JOINT SPACING FROM CORNER OR INTERSECTING WALL. 8.2 LOCATE CONTROL JOINT AT TRANSITION BETWEEN LOAD-BEARING CMU WALL AND NON-LOAD-BEARING CMU WALL. 8.3 LOCATE CONTROL JOINT AT TRANSITION BETWEEN WALL SUPPORTED ON SPREAD FOOTINGS AND WALL SUPPORTED ON THICKENED SLAB FOOTINGS. 8.4 LOCATE CONTROL JOINT AT ABRUPT CHANGE IN WALL HEIGHT OR THICKNESS. 8.5 LOCATE CONTROL JOINT AT TRANSITION BETWEEN INTERIOR WALL AND EXTERIOR WALL. 8.6 COORDINATE LOCATION OF CONTROL JOINT IN CMU WALL SUPPORTED ON THICKENED SLAB FOOTING SO THAT SLAB CONTROL JOINTS AND WALL CONTROL JOINTS ALIGN. 8.7 DO NOT LOCATE CONTROL JOINT WITHIN 12" OF CENTERLINE OF BEAM BEARING ON WALL. 8.8 CONTROL JOINTS IN CMU WALLS SHALL BE CONTINUOUS FROM TOP OF FOUNDATION TO TOP OF WALL UNLESS SPECIFICALLY NOTED. 8.9 AT CONTROL JOINTS IN CMU WALLS, TERMINATE HORIZONTAL JOINT REINFORCING 2" CLEAR EACH SIDE OF JOINT. 8.10 AT CONTROL JOINTS IN CMU WALLS, HORIZONTAL BOND BEAM REINFORCING SHALL BE TERMINATED 2" CLEAR EACH SIDE OF JOINTS EXCEPT AS FOLLOWS: A. 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3. NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS.	7. FOUNDATIONS 1. FOUNDATION DESIGN IS BASED ON THE RECOMMENDATIONS IN THE GEOTECHNICAL REPORT PREPARED BY S&ME, INC., REPORT NUMBER 1611-10-4269v1 DATED OCTOBER 17, 2011 (REFERRED TO HEREIN AS GEOTECHNICAL REPORT). STEVENS & WILKINSON IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD DIFFERENT FROM THOSE ASSUMED FOR DESIGN. 2. THE OWNER'S STRUCTURAL TESTING/INSPECTION AGENCY SHALL VERIFY THAT THE BEARING MEDIUM MEETS THE RECOMMENDATIONS OUTLINED IN THE GEOTECHNICAL REPORT PRIOR TO PLACING REINFORCING OR CONCRETE OR ANY REQUIRED STRUCTURAL FILL. 3. ALL EXCAVATION AND BACKFILL PROCEDURES AND MATERIALS SHALL CONFORM TO RECOMMENDATIONS OUTLINED BY THE GEOTECHNICAL REPORT. THE OWNER'S TESTING AGENCY SHALL INSPECT AND VERIFY ALL PROCEDURES AND MATERIALS ARE CONSISTENT WITH THE REPORT. 4. TEMPORARY DEWATERING SHALL BE PROVIDED AS REQUIRED FOR CONSTRUCTION OF THE FOUNDATIONS AND SLABS ON GRADE. 5. MAT FOOTINGS UNDER SHEAR WALLS AND INDIVIDUAL SPREAD FOOTINGS WITHIN THE BASEMENT AREAS SHALL BEAR ON SOIL CAPABLE OF SUPPORTING 5000 PSF. INDIVIDUAL FOOTINGS OUTSIDE OF THE BASEMENT AREAS SHALL BEAR ON SOIL CAPABLE OF SUPPORTING 3000 PSF. 6. RETAINING WALL AND OTHER CONTINUOUS FOOTINGS SURROUNDING THE BASEMENT AND AUDITORIUM AREAS SHALL BEAR ON SOIL CAPABLE OF SUPPORTING 5000 PSF. ALL OTHER RETAINING WALL AND CONTINUOUS FOOTINGS SHALL BEAR ON SOIL CAPABLE OF SUPPORTING 3000 PSF. 7. 5.1 NO FOOTINGS SHALL BEAR ON ROCK UNDERCUT ROCK A MINIMUM OF 2 FEET BELOW BOTTOM OF FOOTING AND REPLACE WITH STRUCTURAL FILL. 8. FOUNDATION WALLS ARE DESIGNED FOR LATERAL PRESSURES DUE TO THE FOLLOWING EQUIVALENT FLUID DENSITIES: WALLS SUPPORTED AT TOP (AT-REST CONDITION): 60 PCF WALLS FREE TO DISPLACE AT TOP (ACTIVE CONDITION): 40 PCF 9. 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ALL OVERLAPPING CONCRETE QUANTITIES ARE TO BE ANTICIPATED AND INCLUDED IN THE CONTRACTOR'S BID. THE CONTRACTOR SHALL COORDINATE EMBEDDED ITEMS REQUIRED FOR ARCHITECTURAL, STRUCTURAL, AND MECHANICAL ELEMENTS. CONCRETE FLOORS UTILIZING UNSHORED CONSTRUCTION SHALL BE SCREEDED LEVEL. 6.2 STUD LAYOUT SHALL BE PER AISC CODE, CHAPTER 1. 6.3 HEADED SHEAR STUD CONNECTORS, WELDING, AND TESTING SHALL CONFORM TO THE REQUIREMENTS OF STRUCTURAL WELDING CODE - STEEL A53 D1.1. 6.4 TOP FLANGE OF STRUCTURAL STEEL BEAMS AND SUPPORTS TO RECEIVE STUDS SHALL BE FREE OF PAINT, SCALE, RUST, AND OTHER SUBSTANCES WHICH WOULD BE DETRIMENTAL TO THE WELDING OF STUDS THROUGH DECK. 6.5 WHERE A CLOSURE PLATE OR MISCELLANEOUS STEEL MEMBER IS WELDED TO THE TOP FLANGE OF BEAM TO RECEIVE SHEAR STUDS, ATTACH STUDS DIRECTLY TO BEAM FLANGE AND NOT TO (OR THROUGH) PLATES OR MISCELLANEOUS MEMBER. 6.6 REMOVE ALL FERRELS AND DEBRIS FROM DECK PRIOR TO SLAB PLACEMENT. 6.7 PROVIDE D2L ANCHORS WHERE INDICATED IN SECTIONS AND DETAILS. "STICK WELDING" OF REINFORCING IS NOT PERMITTED AS AN ALTERNATE.
6. MECHANICAL, ELECTRICAL, PLUMBING, AND ENCLOSURE RELATED WORK WILL BE SHOWN IN A SUBSEQUENT BID PACKAGE (BP3). RELEVANT DRAWINGS AND DETAILS WILL BE MADE AVAILABLE AFTER BID AWARD. THIS WORK SHALL BE COORDINATED WITH THE STRUCTURAL DRAWINGS BY THE CONTRACTORS OF EACH BID PACKAGE AND THE CONSTRUCTION MANAGER. THIS WORK INCLUDES BUT IS NOT LIMITED TO THE FOLLOWING: 6.1 APPROXIMATELY 300 ELECTRICAL AND AUDIOVISUAL FLOOR BOXES DISTRIBUTED OVER LEVELS 1, 3, AND 4 (EXACT DISTRIBUTION AND PRECISE LOCATION WILL BE PROVIDED IN BP3 DOCUMENTS).	10.1 CAST-IN-PLACE CONCRETE REINFORCEMENT COVER PERMANENTLY EXPOSED TO EARTH: CAST AGAINST THE EARTH 3" CLEAR EXPOSED TO EARTH OR WEATHER: FOR BARS LARGER THAN A NO. 5 BAR: 2" CLEAR NO. 5 BARS OR SMALLER 1 1/2" CLEAR NOT EXPOSED TO WEATHER OR IN CONTACT WITH EARTH: WALLS 3/4" CLEAR SLABS 3/4" CLEAR COLUMN TIES 1 1/2" CLEAR BEAM/GIRDER STIRRUPS 1 1/2" CLEAR	CONCRETE MASONRY 1. CONCRETE MASONRY WORK SHALL CONFORM TO ACI 530, BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES AND ACI 530.1, SPECIFICATION FOR MASONRY STRUCTURES. 2. MINIMUM COMPRESSIVE STRENGTH OF CONCRETE MASONRY SHALL BE FM = 1,500 PSI. 3. CONCRETE MASONRY UNITS SHALL CONFORM TO THE FOLLOWING: 3.1 LOAD BEARING AND NON-LOAD BEARING MASONRY WALLS: ASTM C90 HOLLOW UNITS 3.2 CONCRETE BUILDING BRICK: ASTM C57 4. MORTAR SHALL CONFORM TO ASTM C270: 4.1 TYPE "S" MORTAR, UNO. 5. GROUT FOR ALL MASONRY SHALL CONFORM TO ASTM C476. SUBMIT GROUT MIX DESIGNS INCLUDING MANUFACTURER'S CERTIFICATION FOR MATERIALS USED PRIOR TO THE START OF ANY MASONRY WORK. 5.1 PROVIDE FINE GROUT IN GROUT SPACES LESS THAN 2" IN ANY HORIZONTAL DIMENSION OR WHERE CLEARANCE BETWEEN REINFORCING AND MASONRY IS LESS THAN 3/4". 5.2 PROVIDE COURSE GROUT IN SPACES 2" OR GREATER IN ALL HORIZONTAL DIMENSIONS PROVIDED THE CLEARANCE BETWEEN REINFORCING AND MASONRY IS GREATER THAN 3/4". 5.3 THE MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS FOR ALL GROUT SHALL BE 3,000 PSI, UNLESS NOTED OTHERWISE. 6. ALL TESTING AND INSPECTION OF MASONRY WORK SHALL BE AS SPECIFIED IN ACI 530.1. 6.1 WALLS: INSPECT EACH SECTION OF WALL AND VERIFY REINFORCEMENT PLACEMENT PRIOR TO GROUTING OPERATIONS. VERIFY THAT VERTICAL CELLS AND BOND BEAMS TO RECEIVE GROUT ARE CLEANED OUT TO RECEIVE GROUT. 6.2 GROUT: OBSERVE GROUT OPERATIONS TO INSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS. 7. FOR BRICK EXPANSION AND CONTROL JOINTS SEE ARCHITECTURAL DRAWINGS. 8. CMU CONTROL JOINTS SHALL BE LOCATED USING THE FOLLOWING GUIDELINES: 8.1 UNLESS SPECIFICALLY NOTED, CONTROL JOINT SPACING IN STRAIGHT WALLS SHALL BE THE LESSER OF ONE-AND-ONE-HALF TIMES THE WALL HEIGHT (1-1/2 x H) AND 25' AT CORNERS AND INTERSECTIONS. LOCATE CONTROL JOINT WITHIN ONE-HALF (1/2) STRAIGHT WALL JOINT SPACING FROM CORNER OR INTERSECTING WALL. 8.2 LOCATE CONTROL JOINT AT TRANSITION BETWEEN LOAD-BEARING CMU WALL AND NON-LOAD-BEARING CMU WALL. 8.3 LOCATE CONTROL JOINT AT TRANSITION BETWEEN WALL SUPPORTED ON SPREAD FOOTINGS AND WALL SUPPORTED ON THICKENED SLAB FOOTINGS. 8.4 LOCATE CONTROL JOINT AT ABRUPT CHANGE IN WALL HEIGHT OR THICKNESS. 8.5 LOCATE CONTROL JOINT AT TRANSITION BETWEEN INTERIOR WALL AND EXTERIOR WALL. 8.6 COORDINATE LOCATION OF CONTROL JOINT IN CMU WALL SUPPORTED ON THICKENED SLAB FOOTING SO THAT SLAB CONTROL JOINTS AND WALL CONTROL JOINTS ALIGN. 8.7 DO NOT LOCATE CONTROL JOINT WITHIN 12" OF CENTERLINE OF BEAM BEARING ON WALL. 8.8 CONTROL JOINTS IN CMU WALLS SHALL BE CONTINUOUS FROM TOP OF FOUNDATION TO TOP OF WALL UNLESS SPECIFICALLY NOTED. 8.9 AT CONTROL JOINTS IN CMU WALLS, TERMINATE HORIZONTAL JOINT REINFORCING 2" CLEAR EACH SIDE OF JOINT. 8.10 AT CONTROL JOINTS IN CMU WALLS, HORIZONTAL BOND BEAM REINFORCING SHALL BE TERMINATED 2" CLEAR EACH SIDE OF JOINTS EXCEPT AS FOLLOWS: A. BOND BEAM REINFORCING NEAREST EACH FLOOR OR ROOF LEVEL SHALL BE CONTINUOUS ACROSS CONTROL JOINTS. B. WHERE THE FLOOR-TO-FLOOR OR FLOOR-TO-ROOF HEIGHT BETWEEN BOND BEAMS EXCEEDS 12'-0", REINFORCING IN ONE INTERMEDIATE BOND BEAM APPROXIMATELY MID-HEIGHT OF WALL SPAN SHALL BE CONTINUOUS ACROSS CONTROL JOINTS. C. BOND BEAM REINFORCING AT TOP OF PARAPET WALLS SHALL BE CONTINUOUS ACROSS CONTROL JOINTS. D. WHERE BOND BEAMS ARE STEPPED TO FOLLOW SLOPED ROOF LINES, BOND BEAM REINFORCING SHALL BE CONTINUOUS ACROSS CONTROL JOINTS. 	